## 21.66 ACRES +/PRINCE GEORGE COUNTY, VA

## **ASKING PRICE ~ \$195,000**



### REPRESENTED BY:

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## TABLE OF CONTENTS

- I. PROPERTY DESCRIPTION
- II. TAX MAP
- III. AERIAL PHOTOGRAPH
- IV. TOPOGRAPHIC MAP
- V. ZONING MAP
- VI. LOCATION MAP
- VII. PRELIMINARY WETLANDS
  DELINEATION
- VIII. SOILS REPORT

## PROPERTY DESCRIPTION

The subject property is shown on Prince George County Tax Map as shown below. A review of the Prince George County Tax Records indicates the following:

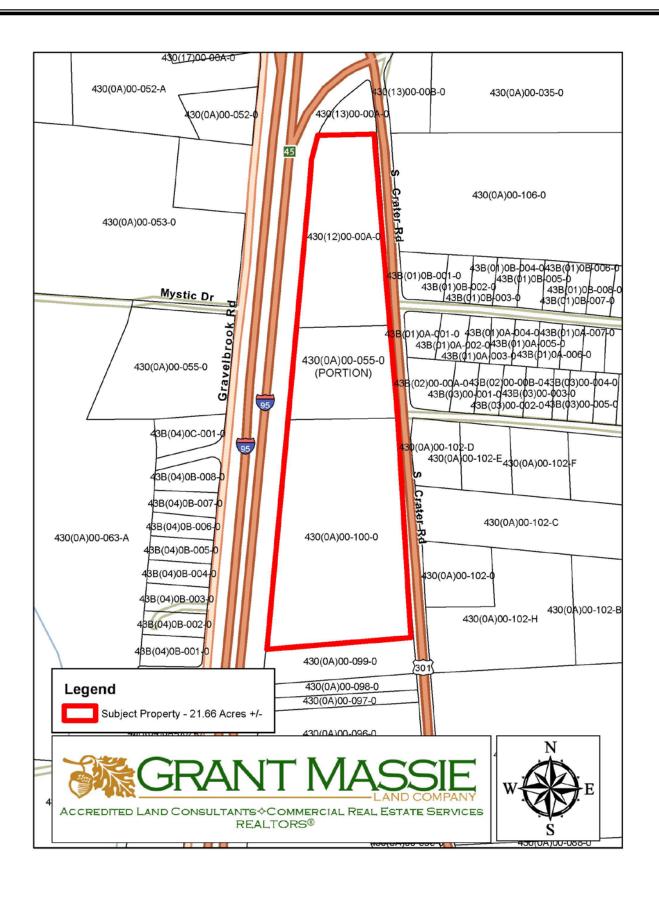
Tax Map Number	<u>Acreage</u>	Tax Assessment
430(0A)00-100-0	12.16	\$262,100
430(0A000-055-0 (Portion)	3.85	\$191,200
430(12)00-00A-0	<u>5.65</u>	<u>\$221,800</u>
	21.66	\$675,100

The subject property is currently zoned B-1(Business). There is long frontage on Route 301 (South Crater Road). The parcels are wooded with some timber value.





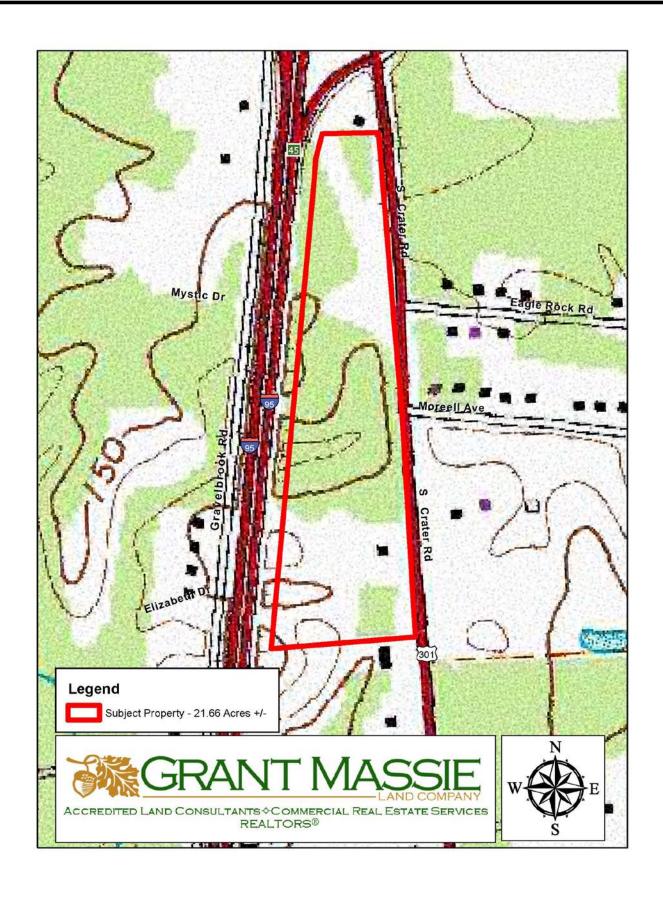
### TAX MAP



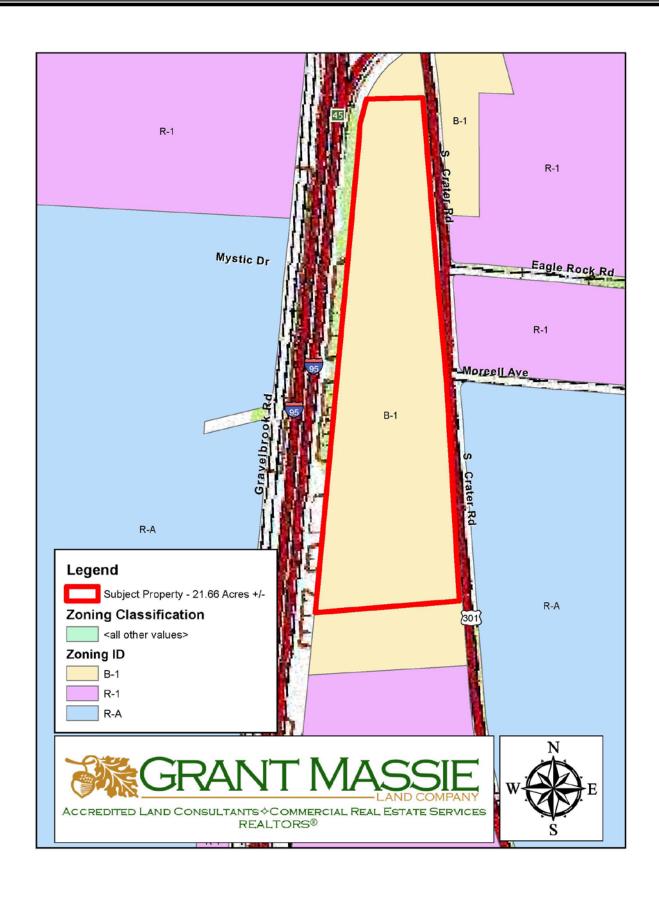
## **AERIAL PHOTOGRAPH**



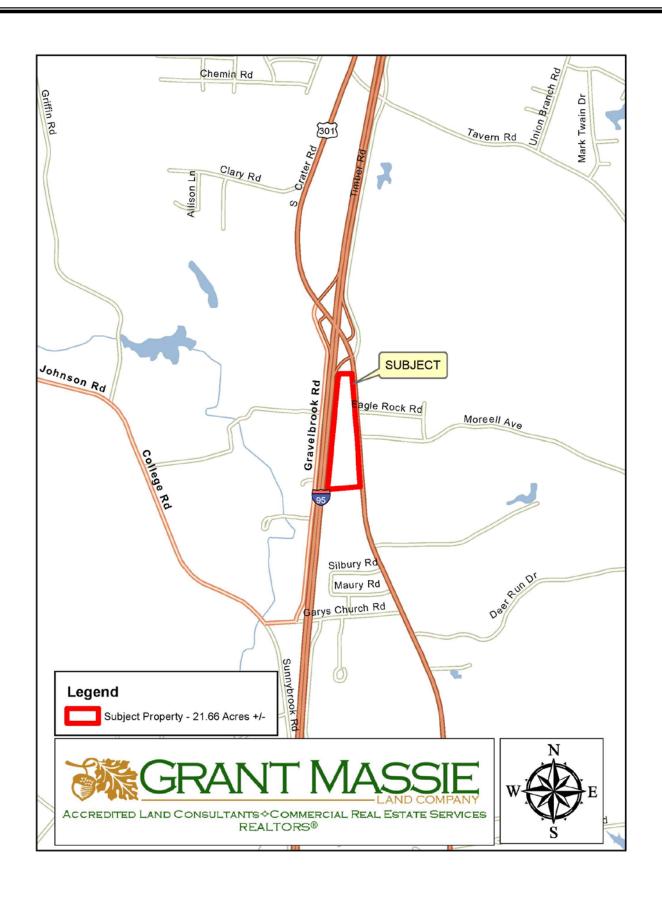
## TOPOGRAPHIC MAP



## **ZONING MAP**

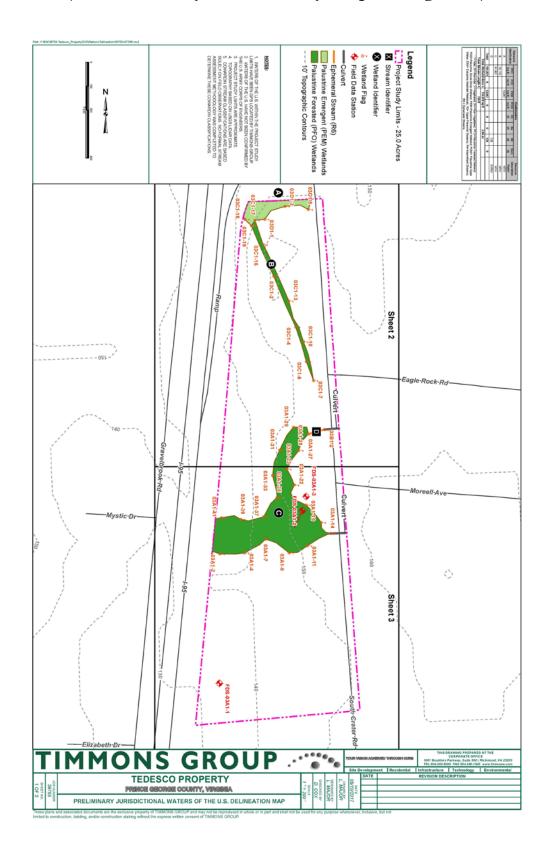


## LOCATION MAP



## PRELIMINARY WETLANDS DELINEATION

(Unconfirmed by the U.S. Army Corps of Engineers)



## SOILS REPORT



# Preliminary Soils Analysis and Infiltration Testing Proposed On-Site Sewage Disposal System TP430(0A)00-100-0, 430(0A)00-055-0 & 430(12)00-00A-0 South Crater Road Prince George County, Virginia

Client: Timmons Group Prince George, Virginia

June 7, 2017



June 7, 2017

Timmons Group Attn: Mr. Derrick Johnson, P.E. 4701 Owens Way; Suite 900 Prince George, Virginia 23875

RE: Onsite Sewage Disposal System Feasibility Study

TP# 430(0A)00-100-0, 430(0A)00-055-0 & 430(12)00-00A-0

South Crater Road

Prince George County, Virginia

Project No. 2017234

Dear Mr. Johnson,

Geo-Solutions is pleased to present the results of our soil texture and infiltration analysis with respect to potential use of portions of the above referenced properties for onsite sewage disposal. You have indicated that zoning would allow up to 6000 gallons per day of wastewater usage for the potential development.

Please do not hesitate to contact Geo-Solutions if you have any questions regarding the findings presented within this report. Geo-Solutions greatly appreciates the opportunity to serve as your soils and onsite sewage consultant on this project and we look forward to a continued successful working relationship.

Sincerely,

**GEO-SOLUTIONS** 

Brent E. Johnson; P.E., P.G., AOSE

President

Shannon D. Hill; P.E., LPSS, AOSE

Project Manager

### **TABLE OF CONTENTS**

- 1.0 INTRODUCTION AND PROJECT SCOPE
- 2.0 SITE DESCRIPTION
- 3.0 ONSITE SOIL EVALUATION AND KSAT RESULTS
- 4.0 CONCLUSIONS
- 5.0 LIMITATIONS

### **APPENDICES**

APPENDIX I Boring Location Figure

APPENDIX II Boring Logs

APPENDIX III KSAT Results

### 1.0 INTRODUCTION AND PROJECT SCOPE

Our scope of services for this project included the review of published soil surveys, maps, and literature generated by the Natural Resources Conservation Service (NRCS) to obtain information regarding the soil properties and potential limiting factors or restrictions with regard to the mapped soil series encountered onsite and how those soils could be applied towards the use of onsite septic and treating effluent. Following a literature and map review, Geo-Solutions conducted a site walkover and investigation to include advancement of soil borings to evaluate soil conditions for soil texture, estimated soil percolation/infiltration rates, and depths to seasonal water table. Saturated conductivity (Ksat) testing was also performed in two locations to obtain actual empirical data for rates at which water infiltrates the soil stratum at potential installation depths. The following report contains our observations, conclusions, and recommendations.

#### 2.0 SITE DESCRIPTION

The area of interest consists of three adjacent parcels identified as Tax Map ID: 430(0A)00-100-0, 430(0A)00-055-0 & 430(12)00-00A-0, totaling in areas of 21.5 Acres. The combined property is located to the west of South Crater Road in Prince George County. The project site is currently wooded and undeveloped.

### 3.0 ONSITE SOIL ELVAUATION AND KSAT RESULTS

On June 1 and June 2, 2017, Geo-Solutions performed soil evaluations and Ksat infiltration testing on the subject parcel for the ability to potentially use the onsite soils as a dispersal media for effluent. The subject parcels were evaluated by Virginia licensed Authorized Onsite Soil Evaluators (AOSE) in accordance with the current regulations of the Virginia Department of Health.

A total of eleven (11) soil borings were advanced on the combined parcels to a depth of five feet utilizing a manual hand auger with a 2 ¾" auger bucket. Soil textures, colors, and depths to seasonal water table were noted and recorded at the time of our evaluation. The locations of the borings were selected based on estimated boundaries between series, as shown on soil maps of Prince George County provided by the NRCS. Boring locations were also advanced in a manner as to obtain a representative sample of soils onsite and to gather information to assist in the determination of the feasibility of using the site for mass drainfields and what capacity of effluent (GPD) the land area may potentially be able to accommodate.

A field log of the soils encountered at the completion of the soil borings was prepared by an AOSE onsite. Field logs completed onsite contain information pertaining to soil classification, texture, and seasonal water table indicators such as redoximorphic features and chroma II depletions. (See Appendix II – Boring Logs).

A total of two (2) infiltration (Ksat) tests were conducted to obtain actual percolation rates of soils onsite. Ksat borings were advanced using a 3" manual soil auger bucket, and testing was performed using a Johnson Permeameter. All testing was done in general accordance with the manufacturer's guidelines. Infiltration testing was focused on the soil series with the most suitability for handling septic effluent and in those areas that met the setbacks to seasonal water table as provided by the Health Department regulations. Below is a table abbreviating the results of our Ksat testing. (See Appendix III – Ksat Testing)

				Rate of
Ksat ID	Borehole ID	USDA Classification	Depth of Ksat Test	Infiltration (minutes per inch)
K-1	BH-9	Sandy Clay Loam	24 inches	~ 90 mpi
K-2	BH-4	Clay Loam	36 inches	~ 110 mpi

### 5.0 CONCLUSIONS

It is our understanding that proposed development may include a commercial or retail facility with up to 6000gpd of wastewater usage. The soils encountered onsite were unsuitable for installation of a conventional septic tank effluent in-ground system due to shallow depth to seasonal groundwater. However, the soils across the vast majority of the site will support a TL2 or TL3 level of pretreated effluent dispersed to a shallow placed drip dispersal system. Based on our onsite soils evaluation and infiltration testing, hydraulic loading rates, and level of treatment, it is our opinion that the subject parcels should be able to service the following gallons per day of residential strength effluent (See table below). If wastewater strengths are elevated, additional treatment and/or larger dispersal areas may be needed. Our field testing indicates that installation depths across the majority of the site should not exceed 12 inches below grade and percolation rates of 90-110mpi should be utilized for sizing purposes.

Zone Location	Area in	Percolation Rate	Hydraulic Loading Rate	Installation Depth	Onsite Sewage Rating	Effluent Capacity Gallons Per Day
Zone #1	15	90-110 mpi	0.12	6-12 inches	Somewhat Limited	10,000+ GPD

It should be noted that typically an area for the primary drainfield as well an area for a reserve drainfield must be identified prior to permitting. Reserve drainfield areas can range from 50% to 100% of the primary design.

Alternative pretreatment systems of this size and type require perpetual maintenance contracts to be upheld by the property owner.

In conclusion, the soils encountered onsite are somewhat limited for installation of an onsite sewage disposal system. The site will require secondary or tertiary pretreatment of all effluent and will require specialized shallow placed pressure dispersal systems. The large nature of the site however, may allow for the dispersal of large volumes up to 6000 gallons per day.

### 6.0 LIMITATIONS

This report was prepared in accordance with generally accepted standards of practice for soil science and soil evaluation services for onsite septic systems. No other warranty, either expressed or implied, is made. This report is not to be reproduced, either in whole or in part, without written consent from Geo-Solutions. Our conclusions and recommendations are based upon information provided to us by others, our site observations, and professional judgment. To the best of our knowledge, information provided by others is true and correct, unless otherwise noted; however, Geo-Solutions is not responsible for the accuracy of information provided by others. Our on-site observations pertain only to specific locations at specific times on specific dates. Our observations and conclusions do not reflect variations in subsurface conditions that may exist between sampling locations, in unexplored areas of the site, or at times other than those represented by our observations. Also, latitude and longitudinal coordinates of boring locations were obtained by a handheld GPS and should be considered approximate and not to the margin of error of survey accuracy.

### **APPENDIX I**

**BORING LOCATION FIGURE** 



**\$\Phi\$** BH-1 HAND AUGER LOCATION

Feasibility Study -Exit 45, Prince George

Date: 6/07/2017 | Project #: 2017234

Geo-Solutions 11901 Old Stage Road Chester, VA 23836



= Unsuitable

= Limited TL-2 or TL-3 Pretreated

**\$\Phi\$** BH-1 HAND AUGER LOCATION

Feasibility Study -Exit 45, Prince George

Date: 6/07/2017 | Project #: 2017234

Geo-Solutions 11901 Old Stage Road Chester, VA 23836

### **APPENDIX II**

**BORING LOGS** 



			CONSULTANTS	
Date of E	valuation: (	5/1 & 2/2017		
		S	OIL EVALUATION REPORT	
Hole #	Horizon	Depth (in)	Description of color, texture, etc.	Texture
				Group
BH-1	A	0-6	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	6-12	10YR 5/3 brown; sandy clay loam; friable	IIb
	EB	12-18	10YR 6/3 pale brown, sandy clay loam; friable	IIb
	Bt1	18-26	10YR 6/6 brownish yellow, very moist, clay loam	III
	Bt2	26-46	10YR 5/6 yellowish brown with 7.5YR 5/6 strong	III
			brown redox at 26"; clay loam	
	Bt3	46-60	10YR 5/6 yellowish brown with 7.5YR 5/6 strong	III
			brown and 2.5YR 5/8 red redox and 10YR 7/1	
			light gray depletions at 46", very moist, clay loam	
BH-2	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	4-12	10YR 5/3 brown; sandy loam; friable	IIa
	Bt	12-48	10YR 5/6 yellowish brown with 7.5YR 5/6 strong	IIb
			brown and 2.5YR 5/8 red redox and 10YR 7/1	
			light gray depletions at 34", very moist, clay loam	
BH-3	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	4-11	10YR 5/3 brown; sandy loam; friable	IIa
	Bt	11-48	10YR 5/6 yellowish brown with 7.5YR 5/6 strong	IIb
			brown and 2.5YR 5/8 red redox and 10YR 7/1	
			light gray depletions at 32", very moist, clay loam;	
			free water at 42"	
BH-4	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	4-12	10YR 5/3 brown; sandy clay loam; friable	IIb
	Bt1	12-42	10YR 5/6 yellowish brown with 10YR 7/4 very	III
			pale brown, clay loam	
	Bt2	42-60	10YR 5/6 yellowish brown with 10YR 7/4 very	
			pale brown, clay loam with 10YR 7/1 light gray	
			depletions at 42"	
BH-5	A	0-2	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	2-10	10YR 6/3 pale brown; sandy clay loam; friable	IIb
	EB	10-20	10YR 7/3 very pale brown, sandy clay loam;	IIb
			friable	
	Bt1	20-26	10YR 6/6 brownish yellow, very moist, sandy clay	IIb
			loam	
	Bt2	26-36	10YR 5/6 yellowish brown with 10YR 6/2 lt.	III



			CONSULTANTS	
			brownish gray depletions at 28", clay loam	
	Bt3	36-60	10YR 6/8 brownish yellow with 7.5YR 5/6 strong	IV
			brown redox and 10YR 7/1 light gray depletions,	
			very moist, clay	
BH-6	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	4-16	2.5YR 4/3 reddish brown; sandy clay loam; friable	IIb
			with 7.5YR 5/4 brown redox	
	Bt1	16-36	10YR 7/4 very pale brown and 7.5YR 6/6 reddish	IIb
			yellow redox, sandy clay loam; friable	
	Bt2	36-60	10YR 7/4 very pale brown and 7.5YR 6/6 reddish	IIb
			yellow redox, sandy clay loam; friable, 10YR 7/1	
			It gray depletions at 36"; Free water at 22"	
BH-7	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	Е	4-14	2.5YR 4/3 reddish brown; sandy clay loam; friable	IIb
			with 7.5YR 5/4 brown redox	
	Bt1	14-35	10YR 7/4 very pale brown and 7.5YR 6/6 reddish	IIb
			yellow redox, sandy clay loam; friable	
	Bt2	35-60	10YR 7/4 very pale brown and 7.5YR 6/6 reddish	IIb
			yellow redox, sandy clay loam; friable, 10YR 7/1	
			It gray depletions at 36"; Free water at 36"	
BH-8	A	0-3	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	EB	3-20	10YR 7/4 very pale brown; light sandy clay loam;	IIb
			friable	
	Bt1	20-26	10YR 6/6 brownish yellow, sandy clay loam	IIb
	Bt2	26-60	10YR 6/8 brownish yellow with 7.5YR 5/8 strong	IIb
			brown redox, sandy clay loam; 10YR 7/1 lt gray	
			depletions at 29"	
BH-9	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	EB	4-18	10YR 7/4 very pale brown; light sandy clay loam;	IIb
	_		friable	-
	Bt1	18-30	10YR 6/6 brownish yellow with 7.5YR 6/8 strong	IIb
	_ • • •		brown redox, sandy clay loam	
	Bt2	30-60	7.5YR 7/8 reddish yellow with 10YR 7/2 light	IIb
	_ <b></b>		gray depletions at 32", sandy clay loam	
			C y are the same and y comments and the same	
BH-10	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	EB	4-18	10YR 7/4 very pale brown; light sandy clay loam;	IIb
	20	1 10	friable	110
		L	1114010	



	Bt1	18-37	10YR 6/6 brownish yellow with 7.5YR 6/8 strong	IIb
			brown redox @ 32", sandy clay loam	
	Bt2	37-60	7.5YR 7/8 reddish yellow with 10YR 7/2 light	III
			gray depletions at 44", clay loam	
BH-11	A	0-4	10YR 4/2 dark grayish brown; sandy loam; friable	IIa
	EB	4-16	10YR 7/4 very pale brown; light sandy clay loam;	IIb
			friable	
	Bt1	16-37	10YR 6/6 brownish yellow with 7.5YR 6/8 strong	IIb
			brown redox @ 33", sandy clay loam	
	Bt2	37-60	7.5YR 7/8 reddish yellow with 10YR 7/2 light	III
			gray depletions at 42", clay loam	

### **APPENDIX III**

**KSAT TESTING** 

	Geo-Solutions	SATURATED HYD	RAULIC CON	JRATED HYDRAULIC CONDUCTIVITY WORKSHEET	ORKSHEE	Ξ		Sheet No.:	
Project Name.:	South Crater Road	Parcel:				Termi	Terminology and Solution	olution	
Boring No:	K-1 next to BH-9		06/02/17		Ksat: Satu	ırated hydraul	Saturated hydraulic conductivity		
Investigators.:	Rainer Haggerty	File Name:			Q: Stea	dy-state rate	Steady-state rate of water flow into the soil	nto the soil	
Boring Depth.:		24" WCU Base. Ht. h:	15.0 cm	cm	H: Con	stant height of	Constant height of water in borehole	hole	
Boring Dia:	3.00	3.00 WCU Susp. Ht. S:	10.0 cm	cm	r: Radi	Radius of cylindrical borehole	al borehole		
Boring Rad. (r):	1.50	1.50 Const. Wtr. Ht. H:	25.0 cm	cm	Ksat = Q[si	nh-1(H/r) - (r²	$= Q[\sinh-1(H/r) - (r^2/H^2+1)^{.5} + r/H] / (2piH^2)$		[Glover R. E.]
VOLUME	Volume Out	Time		Flow Rate Q		Ks	Ksat Equivalent Values	Values	
(m)	(ml) [a]	[min] [b]	0]	(ml/min) [a/b]	(cm/min)	(ces/wo)	(cm/day)	(in/hr)	(ft/day)
1710	290		10.00	29.00	0.019	3.16E-04	27.3	0.448	0.90
1710									
1620	06		10.00	9.00	0.006	9.80E-05	8.5	0.139	0.28
0007									
1620			00 07	00			7	000	0
0/61	90		10.00	00.6	0.003	5.44E-U5	4.7	0.077	01.0
1570									
1530	40		10.00	4.00	0.003	4.36E-05	3.8	0.062	0.12
1530									
1490	40		10.00	4.00	0.003	4.36E-05	3.8	0.062	0.12
1490							C	0	
1450	40		10.00	4.00	0.003	4.36E-05	3.8	0.062	0.12

	Geo-Solutions	SATU	IRATED HYDRAULIC CONDUCTIVITY WORKSHEET	AULIC CON	<b>DUCTIVITY W</b>	ORKSHEE	T		Sheet No.:	
Project Name.:	South Crater Road	Parcel.	· · · · · · · · · · · · · · · · · · ·				Termi	<b>Terminology and Solution</b>	olution	
Boring No:	K-2 next to BH-4	Date	06/02/17	2/17		Ksat: Satu	rated hydraul	Saturated hydraulic conductivity		
Investigators.:	Rainer Haggerty	File Na	File Name:			Q: Stea	dy-state rate	Steady-state rate of water flow into the soil	to the soil	
Boring Depth.:		" wcu B	36" WCU Base. Ht. h:	15.0 <b>cm</b>	sm	H: Cons	tant height of	Constant height of water in borehole	ole	
Boring Dia:	3.0	o wcu s	3.00 WCU Susp. Ht. S:	10.0 <b>cm</b>	sm	r: Radiu	Radius of cylindrical borehole	al borehole		
Boring Rad. (r):	1.5	1.50 Const.	Wtr. Ht. H:	25.0 <b>cm</b>	sm	Ksat = Q[sir	ıh-1(H/r) - (r²,	$= Q[\sinh-1(H/r) - (r^2/H^2+1)^{.5} + r/H] / (2piH^2)$		[Glover R. E.]
VOLUME	Volume Out		Time		Flow Rate Q		Ksa	Ksat Equivalent Values	/alues	
(ml) 2000	(ml) [a]		[a] [uju]		(ml/min) [a/b]	(cm/min)	(cm/sec)	(cm/day)	(in/hr)	(ft/day)
1790	210	0		10.00	21.00	0.014	2.29E-04	19.8	0.324	0.65
1790										
1710	8	80		10.00	8.00	0.005	8.71E-05	7.5	0.123	0.25
1710										
1640	2	70		10.00	7.00	0.005	7.62E-05	9.9	0.108	0.22
1640										
1590	0.00	20		10.00	5.00	0.003	5.44E-05	4.7	0.077	0.15
0661							L 1	0	0	
1560	, n	30		10.00	3.00	0.002	3.27E-05	2.8	0.046	0.09
1590	ď	30		10.00	3 00	2000	3 27E-05	0.8	9700	000
000		<b>D</b>		2	00:5		0.27 1-03	7.0	0.0	50.0